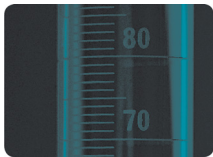


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[https://www.astm.org/DIGITAL\\_LIBRARY/JOURNALS/TESTEVAL/index.html](https://www.astm.org/DIGITAL_LIBRARY/JOURNALS/TESTEVAL/index.html)

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# Journal of Testing and Evaluation

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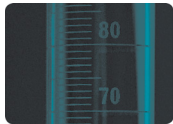
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## Case for Changing Reinforcing Bar Deformation Spacing Requirements

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## Case for Changing Reinforcing Bar Deformation Spacing Requirements

### Reference

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### ABSTRACT

The bond strength of four sets of reinforcing bars is evaluated, two each with No. 5 and No. 10 (No. 16 and No. 32) bars, which have, respectively, nominal diameters of 0.625 and 1.27 in. (15.9 and 32.3 mm). One bar of each size satisfies the criterion for maximum deformation spacing in ASTM reinforcing bar specifications, while the other has deformations that exceed the maximum spacing. All bars exceed the requirements for minimum deformation height. Research related to the effect of deformation properties on bond strength, including the research used to establish the requirements for deformations in ASTM reinforcing bar specifications, is also reviewed. The test results match earlier research and demonstrate that (1) bond strength is not governed by the specific value of deformation height or spacing, but by the combination of the two as represented by the *relative rib area* of the bars and (2) the bond strength of the bars with deformation spacings that exceed those in ASTM reinforcing bar specifications is similar to the bond strength of the bars that meet the specification. Based on this and prior research, it is recommended that ASTM reinforcing bar specifications be modified to allow for deformation spacing up to 90 % (currently a maximum of 70 %) of the bar diameter provided the ratio of deformation height to deformation spacing is greater than or equal to the minimum ratio for bar deformations meeting the current requirements in ASTM reinforcing bar specifications.

### Keywords

bond (concrete to reinforcement), deformed reinforcement, relative deformation area, relative rib area, structural engineering

### Introduction

The deformations on reinforcing bars affect the bond strength between the bars and concrete. ASTM A615, A706, A955, A996, and A1035 [1-5] specify minimum deformation heights and

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24 maximum deformation spacings; however, research has demon-  
 25 strated that it is the relative rib area, a function of the *ratio*  
 26 deformation height to deformation spacing, not the deforma-  
 27 tion height or the deformation spacing alone, that controls  
 28 bond strength.

29 The work presented in this paper, supported by studies  
 30 going back to the 1940s, provides a case for modifying the de-  
 31 formation spacing requirements in ASTM reinforcing bar speci-  
 32 fications [1–5]. The research demonstrates that reinforcing bars  
 33 with deformation spacings exceeding the specified maximums  
 34 provide similar bond strengths to bars with similar relative rib  
 35 areas, regardless of the spacing. It is recommended that the  
 36 specifications be modified to allow for greater deformation  
 37 spacings, provided that the relative rib area of a bar is at least as  
 38 great as it is for reinforcement meeting the minimum require-

39 ments in the current specifications. This will allow for the use of  
 40 a wider range of deformation patterns without the need for  
 41 costly secondary testing and will bring the ASTM specifications  
 42 [1–5] in line with current research regarding the bond strength  
 43 of reinforcing bars.

## 44 Background

45 The requirements for deformation height and spacing in ASTM  
 46 reinforcing bar specifications are based on research by Clark [6,7]  
 47 who observed that the bond capacity of a reinforcing bar increases  
 48 as the ratio of the rib bearing area (projected rib area normal to  
 49 the bar axis) to the shearing area (bar perimeter times distance  
 50 between ribs) increases (Fig. 1). Today, the ratio is most often  
 51 referred to as the “relative rib area,”  $R_r$ , [8] which is expressed as

$$(1) \quad R_r = \frac{\text{projected deformation area normal to bar axis}}{\text{nominal bar perimeter} \times \text{center-to-center deformation spacing}}$$

52 The term “relative deformation area,”  $R_{db}$ , has been adopted in  
 53 ASTM A955 [3].

54 In the case of conventional reinforcing bars that have longi-  
 55 tudinal ribs,  $R_r$  may be calculated as [3,8,9]

$$(2) \quad R_r = \frac{h_r}{s_r} \left( 1 - \frac{\sum \text{gaps}}{p} \right)$$

56 where

57  $h_r$  = average height of deformations, in. or mm,

58  $s_r$  = average spacing of deformations, in. or mm,

59  $\sum \text{gaps}$  = sum of the gaps between ends of deformations,  
 60 plus the width of any continuous longitudinal lines used to rep-  
 61 resent the grade of the bar, multiplied by the ratio of the height  
 62 of the line to  $h_r$ , in. or mm,

63  $p$  = nominal perimeter of the bar, in. or mm.

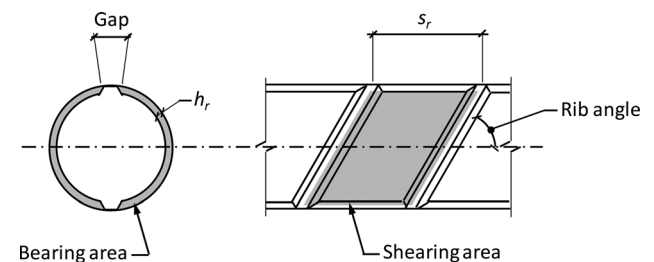
64 Clark [6,7] and other researchers [10–15] have demon-  
 65 strated that  $R_r$ , not the minimum rib height or maximum defor-  
 66 mation spacing, controls the bond strength between reinforcing  
 67 steel and concrete.

68 Rather than including a criterion for  $R_r$  in ASTM standards,  
 69 however, Clark’s study was used to establish a maximum aver-  
 70 age spacing of deformations equal to 70 % of the nominal diam-  
 71 eter of the bar and a minimum height of deformations equal to  
 72 4 % for bars with a nominal diameter of 1/2 in. (13 mm) or  
 73 smaller, 4.5 % for bars with a nominal diameter of 5/8 in.  
 74 (16 mm), 5 % for bars up to a diameter of 1.693 in. (43 mm),  
 75 and 4.5 % for bars with a diameter of 2.257 in. (57.3 mm) [16].  
 76 These provisions remain in use today [1–5], and when com-  
 77 bined with the specified limit on the maximum width of longi-

78 tudinal ribs (equal to 25 % of the nominal perimeter of the bar),  
 79 reinforcing bars meeting the ASTM deformation criteria will  
 80 provide minimum values of  $R_r$  on the order of 0.05, as shown in  
 81 Table 1. In practice, U.S. reinforcing steel typically has values of  
 82  $R_r$  between 0.057 and 0.084 [17].

83 Using specially machined 1 in. diameter bars with values of  
 84  $R_r$  ranging from 0.05 to 0.20 (within and above the typical  
 85 range), Darwin and Graham [12] demonstrated that relative rib  
 86 areas in this range play no role in bond strength for bars not  
 87 confined by transverse reinforcement but do play a role for bars  
 88 confined by transverse reinforcement such as stirrups or ties.  
 89 The results obtained by Darwin and Graham [12] are summar-  
 90 ized in Fig. 2. The figure shows that the bond strength of bars  
 91 confined by transverse reinforcement is principally controlled  
 92 by the relative rib area, which is governed by the combination  
 93 of deformation height and spacing, not by the minimum height  
 94 or the maximum spacing alone. One item worth noting (Fig. 2)  
 95 is that the bars with deformation height  $h = 0.10$  in. (2.5 mm)

FIG. 1 Schematic of reinforcing bar showing deformations (after Ref [8]).



**TABLE 1** ASTM reinforcing bar deformation requirements [1-4].

Bar Designation No.	Nominal Diameter in. (mm)	Deformation			
		Requirements, in. (mm)			
		Maximum Average Spacing	Minimum Average Height	Maximum Sum of Gaps	Minimum Relative Rib Area <sup>a</sup>
3 (10)	0.375 (9.5)	0.262 (6.7)	0.015 (0.38)	0.286 (7.2)	0.043
4 (13)	0.500 (12.7)	0.350 (8.9)	0.020 (0.51)	0.382 (9.8)	0.043
5 (16)	0.625 (15.9)	0.437 (11.1)	0.028 (0.71)	0.478 (12.2)	0.048
6 (19)	0.750 (19.1)	0.525 (13.3)	0.038 (0.97)	0.572 (14.6)	0.054
7 (22)	0.875 (22.2)	0.612 (15.5)	0.044 (1.12)	0.668 (17.0)	0.054
8 (25)	1.000 (25.4)	0.700 (17.8)	0.050 (1.27)	0.776 (19.4)	0.054
9 (29)	1.128 (28.7)	0.790 (20.1)	0.056 (1.42)	0.862 (21.8)	0.053
10 (32)	1.270 (32.3)	0.889 (22.6)	0.064 (1.63)	0.974 (24.8)	0.054
11 (36)	1.410 (35.8)	0.987 (25.1)	0.071 (1.80)	1.080 (27.4)	0.054
14 (43)	1.693 (43.0)	1.185 (30.1)	0.085 (2.16)	1.296 (31.0)	0.054
18 (57)	2.257 (57.3)	1.58 (40.1)	0.102 (2.59)	1.728 (43.8)	0.048

<sup>a</sup>Based on maximum average spacing and minimum average height. Included for reference.

had a deformation spacing of 1 in. (25 mm), equal to one bar diameter and, thus, greater than the value of 70 % of the bar diameter allowed by ASTM, but performed as well as bars with closer deformation spacings. These observations have been shown to be true for conventional reinforcement with a wide range of relative rib areas [13-15]. The role of the relative rib area is now well understood and widely accepted [3,8,9].

The bond test used by Darwin and Graham [12] has been standardized as ASTM A944 “Standard Test Method for Comparing Bond Strength of Steel Reinforcing Bars to Concrete Using Beam-End Specimens” [18]. One application of the test procedure is to qualify coatings of epoxy-coated reinforcement specified in ASTM A775 and A934 [19,20].

In the current study, hot-rolled No. 5 and No. 10 (No. 16 and No. 32) bars were tested for bond strength in accordance with ASTM A944 [17]. For each bar size, the bond strength of bars with a deformation spacing that exceeded the maximum

permitted by ASTM specifications [1-5] was compared with the bond strength of bars that met the spacing requirements. The results match those of earlier tests and demonstrate that the bars with deformation spacings in excess of those currently permitted by ASTM will provide satisfactory bond performance. Full details of the study are reported in Ref. [21].

## Experimental Work

### BAR PROPERTIES

Four sets of reinforcing bars were tested in this study, two each with No. 5 and No. 10 (No. 16 and No. 32) bars. For each set, deformation height and spacing were measured on three bars and the average relative rib area calculated using Eq 2. All bars exceeded the requirements for minimum deformation height. One set of each size satisfied the criterion for maximum deformation spacing, while the other had deformations that exceeded the maximum spacing. The bar properties are summarized in Table 2. All bars had values of relative rib area  $R_r$  that exceeded the minimum values listed in Table 1, with values ranging from 0.070 to 0.084.

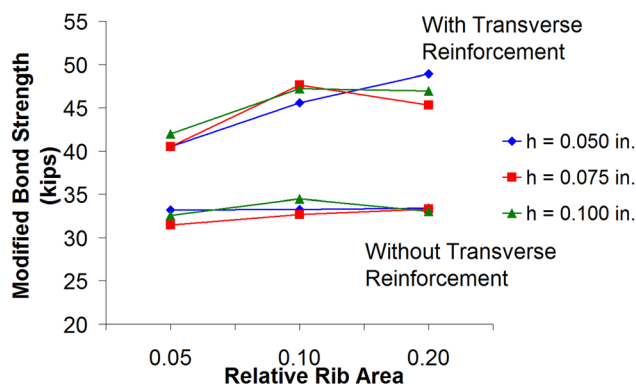
### CONCRETE

Non-air-entrained concrete supplied by a local ready mix plant was used to fabricate the test specimens. The mixture proportions are summarized in Table 3.

### SPECIMEN PREPARATION AND TESTING

The bars were tested, as delivered, with mill scale on the surface. Prior to specimen fabrication, the bar surface was cleaned with acetone to remove any grease or oils. The specimens were prepared and tested in accordance with ASTM A944 [18], as shown in Fig. 3. A summary of specimen properties is presented in

**FIG. 2** Relationship between bond strength and relative rib area for machined bars with heights of deformations equal to 0.05, 0.075, and 0.100 in. (1.27, 11.91, and 2.54 mm) (after Ref. [11]).



**TABLE 2** Properties of bars used in the tests.

Meets Specified Spacing	Bar Designation No.	Nominal Diameter in. (mm)	Deformation			
			Properties, in. (mm)			
			Average Spacing <sup>a</sup>	Average Height	Sum of Gaps	Relative Rib Area
No	5 (16)	0.500 (12.7)	0.440 (11.2)	0.0412 (1.04)	0.312 (7.9)	0.079
Yes	5 (16)	0.500 (12.7)	0.391 (9.9)	0.0377 (0.96)	0.260 (6.6)	0.084
No	10 (32)	1.270 (32.3)	0.901 (22.9)	0.0735 (1.86)	0.564 (14.3)	0.070
Yes	10 (32)	1.270 (32.3)	0.768 (19.5)	0.0656 (1.67)	0.559 (14.2)	0.073

<sup>a</sup>Maximum spacing in accordance with ASTM A615 = 0.437 in. (11.1 mm) for No. 5 (No. 16) bars and 0.889 in. (22.6 mm) for No. 10 (No. 32) bars.

**142 Table 4.** An unbonded lead length (length of bar isolated from  
**143** concrete using PVC pipe) of 1/2 in. (12.7 mm) was used in ac-  
**144** cordance with ASTM A944 [18] to limit the probability of a  
**145** cone-type pullout failure. The embedment lengths ( $l_e$ ) given in  
**146 Table 4** equal the sum of the lead length and the bonded length  
**147** (length of bar in contact with the concrete) of the bar.

**148** Fourteen beam-end specimens were cast and 13 were tested  
**149** for each bar size—seven specimens contained bars that did not  
**150** meet the ASTM deformation spacing requirements [1–5] and  
**151** six specimens contained bars that did. Specimen 1 for the No. 5  
**152** (No. 16) tests and Specimen 13 for the No. 10 bar (No. 32) tests  
**153** were used to verify the functionality of the testing equipment  
**154** and are not used in the comparisons that follow.

**155 Results**

**156 MAXIMUM BOND FORCES**

**157** The maximum bond forces developed by the No. 5 (No. 16) bar  
**158** specimens in the beam-end tests are shown in **Table 5**. The  
**159** mean maximum bond force of the specimens containing the  
**160** No. 5 (No. 16) bars with the deformation spacing that exceeded  
**161** that allowed in ASTM reinforcing bar specifications [1–5] is  
**162** 104.1 % of the mean maximum bond force of the specimens  
**163** containing bars that met the specification. The maximum bond  
**164** forces developed by the specimens with the No. 5 (No. 16) bars  
**165** that did not meet the specifications ranged from 13,106 to  
**166** 17,384 lb (58.3 to 77.3 kN) with a mean value of 16,289 lb,  
**167** standard deviation of 1487 lb (6.6 kN), and coefficient of varia-  
**168** tion of 0.091. The maximum bond forces developed by the  
**169** specimens containing the bars that met the specifications  
**170** ranged from 14,647 to 16,911 lb (65.1 to 75.2 kN), with a mean

value of 15,647 lb (69.6 kN), standard deviation of 849 lb  
 (3.8 kN), and coefficient of variation of 0.054. The mean maxi-  
 mum bond force for the specimens with bars that did not meet  
 specification differs by 642 lb (2.9 kN), less than one standard  
 deviation, from the mean maximum bond force of the speci-  
 mens with the bars that met the specification, indicating little  
 statistical difference between the two.

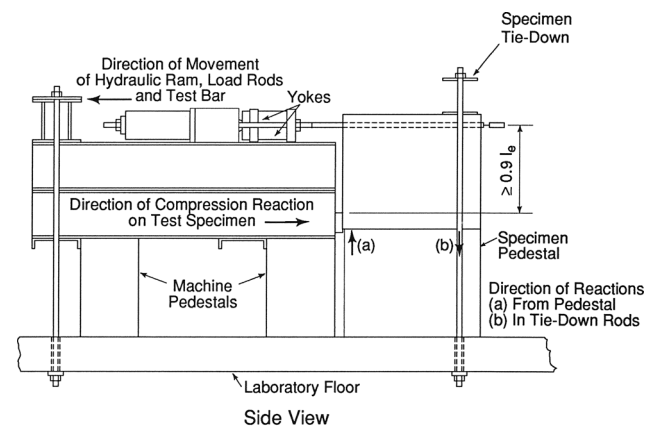
The data were analyzed using the Student’s t-test (used to  
 analyze small data sets). Student’s t-test compares the means  
 and variances of two data sets to determine the probability  $\alpha$   
 that any differences in the mean values could have arisen by  
 chance; that is, that differences in the mean values  $\mu_1$  and  $\mu_2$  are  
 due to natural variability, not differences in the systems. For  
 example,  $\alpha = 0.05$  indicates a 5 % chance that the test will incor-  
 rectly identify (or a 95 % chance of correctly identifying) a stat-  
 istically significant difference in sample means when, in fact,  
 there is no difference. For this analysis, a two-tailed test is per-  
 formed, meaning that there is a probability of  $\alpha/2$  that  $\mu_1$  is  
 greater than  $\mu_2$  and  $\alpha/2$  that  $\mu_1$  is less than  $\mu_2$  when, in fact,  
 $\mu_1$  and  $\mu_2$  are equal.  $\alpha \leq 0.20$  is often used to indicate statistical sig-  
 nificance. Using Student’s t-test for this data set gives  $\alpha = 0.371$ ,  
 further demonstrating that the difference in bond force is not  
 statistically significant.

The maximum bond forces developed by the No. 10 (No.  
 32) test specimens are shown in **Table 6**. The mean maximum

**TABLE 3** Concrete mixture proportions.

Material	Quantity (SSD)
Type I/II cement	564 lb/yd <sup>3</sup> (335 kg/m <sup>3</sup> )
Water	238 lb/yd <sup>3</sup> (141 kg/m <sup>3</sup> )
Kansas river sand	1516 lb/yd <sup>3</sup> (899 kg/m <sup>3</sup> )
Crushed limestone	1709 lb/yd <sup>3</sup> (1013 kg/m <sup>3</sup> )
Estimated air content	1.50 %
Superplasticizer adva 100	28 fl oz (1.08 L)

**FIG. 3** Schematic of test apparatus [17].





**TABLE 4** Specimen properties.

Bar Designation No.	5 (16)	10 (32)
Concrete cover	1–1/4 in. (31.8 mm)	2–5/8 in. (66.7 mm)
Embedment length ( $l_e$ )	8–7/8 in. (225 mm)	14–3/8 in. (365 mm)
Lead length	1/2 in. (12.7 mm)	1/2 in. (12.7 mm)
Moisture condition of concrete during test	Air dry	Air dry
Age at test	12 days	9 days
Compressive strength	5120 psi (35.3 MPa)	5030 psi (34.7 MPa)

196 bond force of the specimens with the No. 10 (No. 32) bars with  
 197 the deformation spacing that exceeded that allowed in ASTM  
 198 specifications [1–5] is 96.4 % of the mean maximum bond force  
 199 of the specimens with bars meeting the specification. The maxi-  
 200 mum bond forces of the specimens with the bars that did not  
 201 meet the specifications ranged from 32,885 to 41,655 lb (146.3  
 202 to 185.3 kN), with a mean of 36,283 lb (161.4 kN), standard  
 203 deviation of 3070 lb (13.7 kN), and coefficient of variation of  
 204 0.085. The maximum bond forces of the specimens containing  
 205 the bars that met the specifications ranged from 32,022 to  
 206 42,929 (142.4 to 202.0 kN), with a mean of 37,653 lb (167.5 kN),  
 207 standard deviation of 4133 lb (18.3 kN), and coefficient of varia-  
 208 tion of 0.110. Like the No. 5 (No. 16) bars, the mean maximum  
 209 bond force for the specimens with bars that did not meet speci-  
 210 fications differs by a relatively small amount, 1370 lb (6.1 kN)  
 211 (again less than one standard deviation), from the mean maxi-  
 212 mum bond force of the specimens with the bars that met the  
 213 specifications, indicating little statistical difference between the  
 214 two values. Analysis using the Student’s t-test,  $\alpha = 0.507$ , also  
 215 indicates that the difference in strength is not statistically  
 216 significant.

**TABLE 5** Maximum bond forces, lb (kN)–No. 5 (No. 16) bars.

Specimen	Meets Specifications	Does Not Meet Specifications
2		16,939 (75.3)
3	15,766 (70.1)	
4		16,837 (74.9)
5	14,748 (65.6)	
6		17,173 (76.4)
7	16,067 (71.5)	
8		16,756 (74.5)
9	15,744 (70.0)	
10		13,106 (58.3)
11	16,911 (75.2)	
12		17,384 (77.3)
13	14,647 (65.1)	
14		15,831 (70.4)
<b>Average</b>	15,647 (69.6)	16,289 (72.5)
<b>Std. Dev</b>	849 (3.8)	1487 (6.6)
<b>COV</b>	0.054	0.091
	Ratio	104.1 %

## Discussion

217

218 The similarity in bond strengths between the bars with deforma-  
 219 tion spacings that exceeded those specified in ASTM A615,  
 220 A706, A955, and A996 [1–5] and those that met the specifica-  
 221 tions is as expected based on the original work by Clark [6,7]  
 222 and subsequent studies [10–15]. Those studies have shown that  
 223 the relative rib area  $R_r$ , not the specific value of deformation  
 224 height or spacing, controls bond strength and that the effect of  
 225  $R_r$  is apparent only when confining transverse reinforcement is  
 226 present, which was not the case in the current tests. The fact  
 227 that the bars in question have values of  $R_r$ , 0.077, and 0.070 for  
 228 the No. 5 and No. 10 bars (No. 16 and No. 32), respectively,  
 229 that exceed the minimum values that result from the ASTM  
 230 provisions [1–5] (Table 1) indicates that these bars will provide  
 231 satisfactory bond performance.

232 The results obtained by Darwin and Graham [12] indicate  
 233 that for a constant  $R_r$ , deformation spacing  $s_r$  may be increased  
 234 up to the diameter of the bar  $d_b$  without affecting bond strength,  
 235 although the following recommendation will be limited to a  
 236 somewhat more conservative value of  $0.9d_b$ . Based on results  
 237 reported here and in prior research [13–15], it is recommended  
 238 that the ASTM reinforcing bar specifications be modified with  
 239 the addition of the following (using ASTM A615 [1] as the  
 240 example):

241 “7.6 The maximum deformation spacing listed in Table 1 (of  
 242 ASTM A615) may be exceeded provided that:

243 7.6.1 The deformation spacing is less than or equal to 90 %  
 244 of the nominal bar diameter, and,  
 245

**TABLE 6** Maximum bond forces, lb (kN)–No. 10 (No. 32) bars.

Specimen	Meets Specifications	Does Not Meet Specifications
1	33,702 (149.9)	
2		33,888 (150.7)
3	32,022 (142.4)	
4		33,727 (150.0)
5	37,726 (167.8)	
6		37,304 (165.9)
7	38,968 (173.3)	
8		36,588 (162.7)
9	42,929 (202.0)	
10		32,885 (146.3)
11	40,571 (190.9)	
12		37,934 (168.7)
14		41,655 (185.3)
<b>Average</b>	37,653 (167.5)	36,283 (161.4)
<b>Std. Dev</b>	4,133 (18.3)	3,070 (13.7)
<b>COV</b>	0.110	0.085
	Ratio	96.4 %

**TABLE 7** (New **Table 2** in ASTM **A615**)—Requirements for bars with high deformation spacing.

Bar Designation No.	Maximum Deformation Spacing, in. (mm)	Minimum Ratio <sup>a</sup>
3 (10)	0.337 (8.5)	0.057
4 (13)	0.450 (11.4)	0.057
5 (16)	0.562 (14.3)	0.064
6 (19)	0.675 (17.2)	0.071
7 (22)	0.787 (20.0)	0.071
8 (25)	0.900 (22.8)	0.071
9 (29)	1.015 (25.8)	0.071
10 (32)	1.143 (29.1)	0.071
11 (36)	1.269 (32.2)	0.071
14 (43)	1.523 (38.7)	0.071
18 (57)	2.031 (51.6)	0.064

<sup>a</sup>Ratio of average deformation height to average deformation spacing.

250 7.6.2 The ratio of deformation height to deformation spacing  
252 is greater than or equal to the minimum ratio presented  
253 in a new Table in ASTM **A615**.  
254

255 The proposed new table for ASTM **A615** is presented as **Table 7**  
256 in this paper.

257 The minimum ratios presented in the proposed table equal  
258 the ratios of the minimum allowable deformation height and  
259 the maximum deformation spacing prescribed in the ASTM  
260 reinforcing bar specifications [1–5] and will result in minimum  
261 relative rib areas equal to those obtained under the current  
262 specifications (shown in **Table 1**). For simplicity, the ratio of deformation  
263 height to deformation spacing is recommended in lieu of the relative rib area.  
264

## 265 Conclusions and Recommendations

266 The following conclusions and recommendations are based on  
267 the results of the tests and analysis presented in this report:

- 268 (1) The test results match earlier research findings and demonstrate that bond strength is not governed by the specific value of deformation height or spacing, but by the combination of the two, as represented by the *relative rib area* of the bars.  
270  
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273
- 274 (2) The bond strengths of the bars with deformation spacings that exceed those specified in the ASTM reinforcing bar specifications are similar to those that meet the specifications. The observed differences in bond strength are not statistically significant.  
276  
277  
278  
279
- 280 (3) The ASTM reinforcing bar specifications should be modified to allow for bar deformations to be spaced up to 90 % of the nominal bar diameter, provided that the minimum ratios of deformation height to deformation spacing based on the current requirements are satisfied.  
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287

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289

## References

290

[1] ASTM Standard **A615/A615M-12**, 2012, “Standard Specification for Deformed and Plain Carbon-Steel Bars for Concrete Reinforcement,” *Annual Book of ASTM Standards*, ASTM International, West Conshohocken, PA. 291  
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293  
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[2] ASTM Standard **A706/A706M-09b**, 2009, “Standard Specification for Low-Alloy Steel Deformed and Plain Bars for Concrete Reinforcement,” *Annual Book of ASTM Standards*, ASTM International, West Conshohocken, PA. 295  
296  
297  
298

[3] ASTM Standard **A955/A955M-12e1**, 2012, “Standard Specification for Deformed and Plain Stainless-Steel Bars for Concrete Reinforcement,” *Annual Book of ASTM Standards*, ASTM International, West Conshohocken, PA. 299  
300  
301  
302

[4] ASTM Standard **A996/996M-09b**, 2009, “Standard Specification for Rail-Steel and Axle-Steel Bars for Concrete Reinforcement,” *Annual Book of ASTM Standards*, ASTM International, West Conshohocken, PA. 303  
304  
305  
306

[5] ASTM Standard **A1035/1035M-11**, 2011, “Standard Specification for Deformed and Plain, Low-carbon, Chromium, Steel Bars for Concrete Reinforcement,” *Annual Book of ASTM Standards*, ASTM International, West Conshohocken, PA. 307  
308  
309  
310  
311

[6] Clark, A. P., “Comparative Bond Efficiency of Deformed Concrete Reinforcing Bars,” *ACI J.*, Vol. 43, No. 4, 1946, pp. 381–400. 312  
313  
314

[7] Clark, A. P., “Bond of Concrete Reinforcing Bars,” *ACI J.*, Vol. 46, No. 3, 1949, pp. 161–184. 315  
316

[8] ACI Committee 408, “Bond and Development of Straight Reinforcing Bars in Tension,” *ACI 408R-03*, American Concrete Institute, Farmington Hills, MI, 2003. 317  
318  
319

[9] ACI Committee 408, “Guide for Lap Splice and Development Length of High Relative Rib Area Reinforcing Bars in Tension and Commentary,” *ACI 408.3R-09*, American Concrete Institute, Farmington Hills, MI, 2009. 320  
321  
322  
323

[10] Soretz, S., and Holzenbein, H., “Influence of Rib Dimensions of Reinforcing Bars on Bond and Bendability,” *ACI J.*, Vol. 76, No. 1, 1979, pp. 111–127. 324  
325  
326

[11] Kimura, H., and Jirsa, J. O., “Effects of Bars Deformation and Concrete Strength on Bond of Reinforcing Steel to Concrete,” Report No. ■, Univ. of Texas at Austin, 1992. 327  
328  
329  
330

[12] Darwin, D., and Graham, E. K., “Effect of Deformation Height and Spacing on Bond Strength of Reinforcing Bars,” *ACI Struct. J.*, Vol. 90, No. 6, 1993, pp. 646–657. 331  
332  
333

[13] Darwin, D., Tholen, M. L., Idun, E. K., and Zuo, J., “Splice Strength of High Relative Rib Area Reinforcing Bars,” *ACI Struct. J.*, Vol. 93, No. 1, 1996a, pp. 95–107. 334  
335  
336

[14] Darwin, D., Zuo, J., Tholen, M. L., and Idun, E. K., “Development Length Criteria for Conventional and High Relative Rib Area Reinforcing Bars,” *ACI Struct. J.*, Vol. 93, No. 3, 1996b, pp. 347–359. 337  
338  
339  
340

[15] Zuo, J., and Darwin, D., “Splice Strength of Conventional and High Relative Rib Area Bars in Normal and High Strength Concrete,” *ACI Struct. J.*, Vol. 97, No. 4, 2000, pp. 630–641. 341  
342  
343  
344

AQ3



- 345 [16] ASTM Standard A305-49, 1949, "Specification for Mini-  
 346 mum Requirements for the Deformations of Deformed  
 347 Steel Bars for Concrete Reinforcement," *Annual Book of*  
 348 *ASTM Standards*, American Society for Testing and Mate-  
 349 rials, Philadelphia, PA.
- 350 [17] Choi, O. C., Hadje-Ghaffari, H., Darwin, D., and McCabe,  
 351 S. L., "Bond of Epoxy-Coated Reinforcement to Concrete:  
 352 Bar Parameters," *SL Report No. 90-1*, Univ. of Kansas Cen-  
 353 ter for Research, Lawrence, KS, 1990.
- 354 [18] ASTM Standard A944-10, 2010, "Standard Test Method  
 355 for Comparing Bond Strength of Steel Reinforcing Bars to  
 356 Concrete Using Beam-End Specimens," *Annual Book of*  
 357 *ASTM Standards*, ASTM International, West Consho-  
 358 hocken, PA.
- [19] ASTM Standard A775/A775M-07b, 2007, "Standard Speci- 359  
 fication for Epoxy-Coated Steel Reinforcing Bars," *Annual* 360  
*Book of ASTM Standards*, ASTM International, West Con- 361  
 shohocken, PA. 362
- [20] ASTM Standard, A934/A934M-07, 2007, "Standard 363  
 Specification for Epoxy-Coated Prefabricated Steel Rein- 364  
 forcing Bars, (ASTM A934/A934M-07)," *Annual Book of* 365  
*ASTM Standards*, ASTM International, West Consho- 366  
 hocken, PA. 367
- [21] Darwin, D., Browning, J., O'Reilly, M., and Xing, L., "Bond 368  
 Strength of Reinforcing Bars with Deformation Spacings 369  
 that Exceed Maximum Specified in ASTM A615," *SL* 370  
*Report No. 08-1*, Univ. of Kansas Center for Research, Inc., 371  
 Lawrence, KS, 2008. 372

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